MULTI-STORY LIGHT FRAME BUILDING - HAZARDS



CHARACTERISTICS

- Mostly wood frame, box type up to 4 stories.
- Residential or Light Commercial.

KEY PERFORMANCE ASPECTS

- Many walls create redundant structures w/ductile failure modes, dependant on sheathing type.
- Presence of concrete floor fill can enhance possibility of P-delta collapse.

TYPICAL FAILURE MODES

- Failure in Wall Sheathing Racking of Walls.
- Failure should be slow and noisy.
- Soft/Weak stories can rack and collapse.

COMMON COMBINATIONS

Many are built over R/C parking garages.

MULTI-STORY LIGHT FRAME BUILDING (continued)

EXPECTED PERFORMANCE – for the following:

- Progressive Collapse Extensive connection failures. Members & components are likely to remain intact.
- **E. Quake** Generally good performance common failure is ductile racking of first story. Raked stories are subject to ratcheting and P-delta collapse in Aftershocks.
- **Explosion** Walls become disconnected from floors (horizontal diaphragms), leading to part or total collapse.
- Fire Rapid combustion and collapse unless fire resistant.
- **High Energy Impact** Little resistance to collapse in immediate area. Remainder of structure remains stable.
- Wind Damage is highly dependent on wind speed vs. shape and proper detailing. Tornados can destroy even well constructed wood buildings.
- Struct Overload/Defect Roof failures due to snow, especially on longer span roofs.

CHECK POINTS

- Badly cracked and/or leaning walls.
- Leaning first story in multi-story buildings.
- Cracked, leaning/loose veneer or chimney.
- Offset of building from foundation.
- Separated porches, split level floors/roof.
- Connection failures nail pullout/bolt pull-through.

HAZARD REDUCTION

- Shut off gas and reduce other fire hazards.
- Avoid or pull-down damaged veneer and chimneys.
- Place vertical and/or lateral (diagonal) shores.
- Monitor changes in racked/leaning structures.

- Vertical access through floor/roof from above collapsed area.
- Horizontal entry through existing cavities, or through walls.
- Remove or shore hazards near victims, if required.

HEAVY WALL- URM BUILDING - HAZARDS



CHARACTERISTICS

- URM Ext walls, wood floors/roof box type to 8 stories.
- Lack of wall strap anchors Red Brick & CMU low-raise.
- Residential, Commercial and Industrial occupancies.

KEY PERFORMANCE ASPECTS

- Walls Brittle with little resistance to unanticipated loads.
- Redundant interior walls may prevent floor collapse.

TYPICAL FAILURE MODES

- Walls separate from roof/floors, leading to falling walls and collapsed roof/floors.
- Cracked/pealed walls create brittle falling hazards.

- Heavy timber, light frame walls & floors.
- Steel joist floors w/concrete fill in multi-story buildings.

HEAVY WALL- URM BUILDING (continued)

EXPECTED PERFORMANCE – for the following:

- **Progressive Collapse** URM walls likely to disintegrate, and interior structure may stand independently.
- E. Quake Poor performance out of plane ext wall failures, loss of connection to floors leading to partial or total collapse. Many lethal Aftershock falling and collapse hazards.
- **Explosion** Walls become disconnected from floors (horizontal diaphragms), leading to part or total collapse.
- Fire Loss of roof/floors will leave walls unbraced. Collapsing roof/floors can thrust walls in or out.
- High Energy Impact Ext URM walls disintegrate upon impact leaving lethal falling hazards & possible floor collapse. Massive masonry is more resistant.
- Wind Roof vulnerable to uplift, leading to partial or total collapse or roof & walls. Massive masonry is more resistant.
- Struct Overload/Defect Roof failures due to ponding and snow. Wood decay, brick disintegration or remodeling in older buildings.

CHECK POINTS

- Loose, broken parapets and ornamentation.
- Connections between exterior walls and roof/floors.
- Cracked wall corners and openings, plus peeled walls.
- Unsupported and partly collapsed roof/floors.

HAZARD REDUCTION

- Shut off gas and reduce other fire hazards.
- Diagonally shore. tie-back, avoid, remove hazardous walls.
- Shore hazardous roof/floor beams, etc.
- Monitor changes in racked/leaning structures.

- Vertical access through floor/roof from above collapsed area.
- Horizontal entry through existing cavities and openings.
- Remove bricks by hand, excavator, or crane w/clamshell.
- Remove or shore hazards near victims, if required.

US&R STRUCTURES SPECIALIST FOG DISASTER SITE REFERENCE DATA **HEAVY WALL- TILT-UP BUILDING - HAZARDS** CHECK TRUSSES FOR BROKEN CONNECTIONS AT BOLTED JOINTS ESPECIALLY AT LOWER CHORDS CHECK FOR SEPARATION OF ROOF BEAMS/PURLINS AT INTERIOR CONNECTIONS CHECK ALL CONNECTIONS BETWEEN EXTERIOR WALLS AND ROOF MEMBERS CHECK HINGE CONN. FOR SPLITS & SLIP CHECK BEAM JOINT AT INTERIOR COLUMN CHECK TOP OF WALL FOR TENSION CHECK BADLY CRACKED WALL PIERS & DOOR HEAD AREAS LOOK FOR OUTWARD CHECK FOR CRACKED LEANING PANELS COLUMNS BETWEEN OPENINGS

CHARACTERISTICS

- Conc. ext walls, wood floors/roof, some steel fl w/concrete fill.
- Long span roof (50ft+) and floors (25ft+).
- Similar performance with CIP conc. or reinforced CMU walls.
- Office, Commercial & Lt Industrial occupancies to 4 stories.

KEY PERFORMANCE ASPECTS

- Robust ext walls, but may have weak connection to roof.
- Post 1995 and retrofit building should perform better.

TYPICAL FAILURE MODES

 Walls separate from roof/floors, leading to falling walls and collapsed roof/floors. Long span collapse is probable.

- Light frame walls & floors 1.5"concrete fill on floors.
- Steel joist, long span floors w/concrete fill.

HEAVY WALL- TILT UP BUILDING (continued)

EXPECTED PERFORMANCE – for the following:

- Progressive Collapse Out-leaning wall/walls could progress to roof/floor collapse in bay adjacent to exterior. Remainder could stand independently – but poorly braced.
- E. Quake Pre 1995 poor performance out of plane ext wall failures, loss of connection to roofs leading to partial or total collapse. Lethal Aftershock falling and collapse hazards.
- Explosion Walls become disconnected from floors (horizontal diaphragms), leading to part or total collapse
- **Fire** Loss of roof/floors will leave walls unbraced. Collapsing roof/floors can thrust walls in or out.
- High Energy Impact Impact on exterior walls likely to be localized. Could lead to localized roof/floor collapse.
- Wind Roof vulnerable to uplift, leading to partial or total collapse or roof and walls. Penetration through large doors can lead to critical uplift and blow-out pressures.
- Struct Overload/Defect Roof failures due to ponding and snow. Wood decay in older buildings.

CHECK POINTS

- Connections between exterior walls and roof/floors.
- Beam to beam and other interior roof connections.

HAZARD REDUCTION

- Diagonal or Raker shore concrete walls.
- Shore hazardous roof/floor beams, etc.
- May pull-down leaning walls after dealing w/roof support.
- Monitor changes in racked/leaning structures.

- Vertical access through floor/roof from above collapsed area. Horizontal entry through existing cavities and openings.
- Cut holes in wall panels, 2 feet min. from joints.
- Remove large wall panels and roof sections by crane.

PRECAST BUILDINGS - HAZARDS



CHARACTERISTICS

- Factory built lightweight concrete parts up to 14 stories.
- Systems w/o interior concrete panels are greatest problem.

KEY PERFORMANCE ASPECTS

- Highly engineered systems, but often brittle connections.
- Little capacity for unanticipated loads.
- Residence type may be highly redundant due to many walls.

TYPICAL FAILURE MODES

• Failure of interconnections between parts leading to partial or total collapse, depending on redundancy.

- May have CIP floor slabs or reinforced concrete topping.
- Use of Reinforced Masonry shear walls and metal stud walls.
- PC is used as floor panels in masonry & steel buildings.

PRECAST BUILDINGS (continued)

EXPECTED PERFORMANCE – for the following:

- **Progressive Collapse** Failed single story columns have lead to progressive collapse. Heavy elements vs. brittle connections are critical issues. Members retain strength.
- E. Quake Very poor performance except for multi-wall residence buildings. Failed connections lead to partial or total collapse. Aftershock falling, shifting and collapse hazards.
- Explosion Poor performance due to weak-link connections leading to part or total collapse.
- Fire Could cause annealing of tendons and prestress loss.
- **High Energy Impact** Impact on ext elements likely to be localized. Brittle connections could be damaged.
- Wind Unlikely to be damaged by wind. Exterior skin and curtain walls could be damaged/destroyed.
- Struct Overload/Defect Failures in connections, leading to cascading structure failure. Members should retain integrity.

CHECK POINTS

- Beam/column connections, broken welds and cracked corbels.
- Column cracking at top, bottom and wall joints.
- Wall connections at floors, columns and foundation.
- Badly cracked walls and columns plus falling hazards.

HAZARD REDUCTION

- Remove/avoid leaning/hanging, concrete elements.
- Shore damaged roof/floor beams, especially next to bad columns.
- Remove/shore unstable wall and floor elements.
- Monitor changes in racked/leaning structures.

- Vertical access through thin horizontal sections from above.
- Horizontal entry through existing cavities and openings.
- Cut holes in wall panels, 2 feet min. from joints.
- Carefully remove large wall/floor sections by crane.

HEAVY FLOOR BLDGS (CIP non-DUCTILE) - HAZARDS



CHARACTERISTICS

- Cast in Place (CIP) concrete frames and highway structures, – up to 12 stories.
- Few concrete walls, but URM infill in older buildings.
- Eastern US (Western pre 1975) Office & Commercial.

KEY PERFORMANCE ASPECTS

- Brittle failure modes when loaded beyond capacity.
- Post 1975 Ductile Frames in western US have systems that can absorb considerable energy w/o loss of integrity.

TYPICAL FAILURE MODES

- Beam-column joint failure or column shear leading to partial or total collapse.
- Collapse can be partial or complete pancake.

COMMON COMBINATIONS

May have URM and/or metal stud wall partitions.

HEAVY FLOOR BLDGS (CIP non-DUCTILE) (continued)

EXPECTED PERFORMANCE – for the following:

- **Progressive Collapse** Members likely to break into smaller pieces. Rubble piles may shift.
- **E. Quake** Very poor performance Brittle failures of columns and beam/column connections, leading to partial or pancake collapse. Aftershocks cause added collapse, falling hazards and shifting.
- **Explosion** Poor slab performance due to reverse gravity loading can lead to loss of column stability and collapse.
- Fire May cause spalling of concrete cover on all elements.
- High Energy Impact Damage limited to area of impact. Could leave damaged members of questionable strength.
- Wind Unlikely to be damaged by wind. Exterior skin and curtain walls could be damaged/destroyed.
- Struct Overload/Defect –Construction falsework failures most common. Members break into pieces w/poor integrity.

CHECK POINTS

- Beam/column connections above and below floors.
- Badly confined concrete in columns (empty basket).
- Diag. shear cracks in beams and cracking in slabs near cols.
- Attachment of URM walls and other heavy objects.
- Cracks in concrete shear walls and stairs.

HAZARD REDUCTION

- Shore/avoid badly cracked slabs, beams and/or column.
- Shore/avoid overloaded slabs due to punching shear.
- Remove/shore unstable wall and floor elements.
- Monitor changes in racked/leaning structures.

- Vertical access through existing access shafts.
- Vertical access by cutting through slabs from above victims.
- Horizontal entry through existing cavities and openings.
- Cut non-bearing/infill walls after careful assessment.
- Remove large pieces by crane, after rebar has been cut.

HEAVY STEEL FRAME BUILDING - HAZARDS



CHARACTERISTICS

- Heavy "W" steel beam & column framing 2 to many stories.
- Office and Commercial Occupancies, some industrial.

KEY PERFORMANCE ASPECTS

- Normally well engineered, but performance is dependent on ductility of connections. PC floor systems as suspect.
- Welded connections may be subject to brittle failure.
- Diagonally braced frames may have buckled cols or braces.

TYPICAL FAILURE MODES

Connection failure leading to partial collapse. Total collapse is extremely rare.

- May have masonry, precast or metal panel exterior walls.
- CIP floors over metal deck, or PC/CIP directly on steel.

HEAVY STEEL FRAME (continued)

EXPECTED PERFORMANCE – for the following:

- **Progressive Collapse** Rare, since members maintain integrity even with damaged/failed joints.
- **E. Quake** Good performance of frame Failure of diagonal bracing and fracture of welded joints have occurred. Facing, especially PC panels could fall and are danger in Aftershocks.
- **Explosion** Good performance of frame but wall & floor panels could be dislodged. Frame collapse is unlikely.
- **Fire** Plastic deformation of floors and some joint failure. Strength is regained upon cooling. Collapse very rare.
- High Energy Impact Impacted members are severed/destroyed. Connection failures near impact only.
- Wind Frame at low risk Skin, especially glass may be destroyed leading to interior partition failure.
- Struct Overload/Defect Failures during erection and longspan failures are most common. Members maintain integrity with failures at joints.

CHECK POINTS

- Indications of movement plumb corners, stair and nonstructural damage – as clues to potential structure damage.
- Main beam to column connections remove finishes as required.
- Broken PC floor and miscellaneous beam bolt connections.

HAZARD REDUCTION

- Shore beams near damaged or broken connections.
- Remove/avoid/tieback damaged exterior facing.
- Monitor changes in racked/leaning structures.

- Vertical access by cutting through slabs from above victims.
- Horizontal entry through existing cavities & openings.
- Remove or shore hazards near victims, if required.

LIGHT METAL BUILDING - HAZARDS



CHARACTERISTICS

- Light-gage steel, pre-fab metal buildings up to 3 stories.
- Industrial and Commercial Occupancies most 1 story.

KEY PERFORMANCE ASPECTS

- Highly engineered with little redundancy or over-strength.
- Very flexible, especially in lateral direction.

TYPICAL FAILURE MODES

- Weakest Link Behavior loss of sheathing allows buckling, leading to collapse of supporting structure.
- Diagonal rod bracing elongation & joint failure.

- May have masonry, precast or tilt-up exterior walls.
- May have wood or metal interior partitions and mezzanine.

LIGHT METAL BLDGS (continued)

EXPECTED PERFORMANCE – for the following:

- **Progressive Collapse** Joint failure and member buckling could lead to part or complete collapse.
- **E. Quake** Good performance Failure of rod bracing is common, but collapse is rare. Minor aftershock response.
- Explosion Skin blown away, possibly leading to frame/roof collapse. Entire building blown away in some cases.
- **Fire** Rapid loss of strength and collapse due to heating. Long span structure could suddenly collapse.
- High Energy Impact Little resistance to impact. Damage may involve several bays of structure.
- Wind At high risk as skin is blown away, frames/trusses can buckle and collapse. Frames can rack and collapse.
- Struct Overload/Defect Lateral torsion buckling of built-up members. Joint failure and member buckling, leading to part or complete collapse.

CHECK POINTS

- Broken, elongated and/or buckled rod bracing & connections.
- Buckled purlins, truss members, and steel frames.
- Broken and/or elongated bolt connections + anchor bolts.

HAZARD REDUCTION

- Shore and/or diagonally brace racked building frames.
- Remove loose or lightly connected members and sheathing.
- Monitor changes in racked/leaning structures.

- Vertical/Horizontal access by removal or cutting sheathing.
- Horizontal entry through existing cavities and openings.
- Remove or shore hazards near victims, if required.